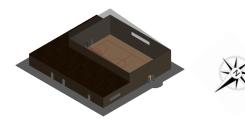


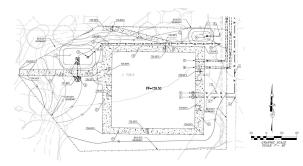
Escondido MFRO Intermediate Booster Pump Station

PRELIMINARY DESIGN



A 3D rendering of our design is shown above. The booster pump station is located in two rooms on the north side of the building along with a concessions stand/office on the north west side. On the east side of the building there are two restrooms, and the basketball court is on the south side of the building. We will propose two biofiltration basins located west of the structure at a low point on the project site.

AUTOCAD DESIGN



Displayed above is our grading design that was developed through AutoCAD Civil 3D. Using the feature line tool, we graded the existing topography for our proposed structure. Our proposed finished floor elevation is 739.50 feet.

PROJECT OVERVIEW

The city of Escondido is constructing a pipeline that conveys water from a water treatment plant to a distribution reservoir. In order to provide enough pressure so that the water can adequately flow from the plant to the reservoir, an intermediate booster pump station must be constructed to support the overall project. Our team at Jump Street Engineering Inc. has designed an intermediate booster pump station along with a

MEET THE SQUAD

recreation center at Mountain View Park to fulfill the needs of the community, and the city of Escondido project requirements.



PROJECT SCHEDULE

Schedule-495				Classis WBS Layout 06-Apr-21 18:02
Activity Name	Supervisor	Start	Finish	Feb 14 Feb 21 Feb 28 Mar 07 Mar 14 Mar 21 Mar 28 Apr 04 Apr 11 Apr 18 Apr 25 Mary 02 Mary 09 / 19
Site Visit	JS	01-Mar-21	01-Mar-21	01-Mar-21A 01-Mar-21A
Zoning Research	ML.	02-Man-21	05-Mar-21	02-Mar-21 A - 05-Mar-21
Soll Classification Research	YC	02-Mar-21	24-Mar-21	02-Mar-21 A + 24-Mar-21
Material Research	AR	02-Mar-21	26-Mar-21	02-Mar-21 A + 26-Mar-21
Watershed Research	EN	02-Mar-21	09-Apr-21	02-Man-21 A + 09-Apr-21
Research Existing Conditions	JS	02-Man-21	22-Mar-21	02-Man-21 A + 22-Man-21
Hydraulic Design Criteria	TF	02-Man-21	12-Mar-21	02-Mar-21 A + 12-Mar-21
Water and Sewer	TF	01-Mar-21	22-Mar-21	01 Mar 21 - 22 Mar 21
Structural Calculations	AR	01-Mar-21	29-Mar-21	01-Mar-21 29-Mar-21
Cost Estimates and Analysis	ML	01-Mar-21	09-Apr-21	01-Mar-21 09-Apr-21
50% Design Submittal	ALL	01-Mar-21	26-Mar-21	01-Mar-21 26-Mar-21
Site Survey / Grading	YC	08-Mar-21	19-Mar-21	08-Mar-21 - 19-Mar-21
Hydraulic Calculations	TF	16-Mar-21	05-Apr-21	16-Mar-21 A - 05-Apr-21
Drainage Report	EN	15-Mar-21	02-Apr-21	15-Mar.21 - 02-Apr.21
Pump Selection	TF	22-Mar-21	26-Mar-21	22-Mar/21 - 26-Mar/21
Proposed Utility Layout	JS	22-Mar-21*	15-Apr-21	224Mar-21* 15-Apr-21
100% Project Submittal	ALL	29-Mar-21	04-May-21	29-Mar/21 04-May/21
Structural Design	AR	30-Mar-21	15-Apr-21	30-Mar-21 + 15-Apr-21
Final Site Layout	AR	16-Apr-21	27-Apr-21	16-Apr-21 27-Apr-21
Presentation Deliverables	ALL	05-May-21	05-May-21	05-May-21 -1 05-May-21
Presentation Video	ALL	05-May-21	05-May-21	05May-21 -1 05May-21

Shown above is a schedule which we created using Primavera P6. The schedule accounts for everything after the proposal, i.e when we were offered the job and shows a 3 month time frame. Early in the schedule, our focus is more towards research and development, later on you can see our focus shifts more towards actualizing our research and getting it pieced together in our 50% submittal, 100% submittal, and other design deliverables.

DESIGN CALCULATIONS & FINDINGS

Electrical Resistivity (ohm-cm)	Chloride Content	Percent Sulfate	РН
Less than 1,100 ohm-cm		More than 0.15%, or 1,500 ppm	Less than 5.5

Field exploration and laboratory testing were performed by Ninyo & Moore

- Major soil: Placentia Sandy Loam (PfC), thick surface, hydrologic soil group D.
- Shallow foundation with spread and continuous footing are recommended.

Total Dynamic Head (P ₂ /Y):	-282.59 ft
Additional Pressure Required (P2):	17633.62 lb/ft ²
Power (Pw):	520.22 BHP

Total dynamic head and additional pressure required were both calculated using Bernouli's equation with an assumed temperature of 70°F.





Marvin Luluquisen

Enrico-Joaquin Santos: Project Manager Axel Rinder: Structural Engineer Yun Chiang: Geotechnical Engineer Marvin Luluguisen: Construction Engineer

Enrico-Joanuin Santos

Tim Fontimayor: Hydraulic Engineer Enrique Naputi: Environmental Engineer



Enrique Naputi



Axel Rinder



Tim Fontimayor



Marvin Luluquisen



Enrico-Joaquin Santos



Yun Chiang

STORM WATER POLLUTION PREVENTION NOTES:

- 1. BEST MANAGEMENT PRACTICES (BMPS) SHALL BE IMPLEMENTED DURING ALL PHASES OF CONSTRUCTION IN CONFORMANCE WITH THE CITY OF ESCONDIDO'S MUNICIPAL CODE. ADDITIONALLY, SITES OVER AN ACRE SHALL ABIDE BY THE CONSTRUCTION GENERAL PERMIT (CGP). ALL BMPS SHALL BE INSTALLED IN ACCORDANCE WITH THE MOST RECENT VERSION OF THE CASQA HANDBOOK. AT A MINIMUM PERIMETER CONTROL AND CONSTRUCTION ENTRANCES SHOULD BE IN PLACE PRIOR TO A GRADING PERMIT BEING ACTIVATED.
- INSPECTION, MODIFICATION AND MAINTENANCE OF THE BMPS SHALL BE IMPLEMENTED AS NECESSARY. IN THE EVENT OF FAILURE OR REFUSAL TO PROPERLY MAINTAIN THE BMPS. THE CITY MAY ISSUE EMERGENCY MAINTENANCE WORK TO BE COMPLETED TO PROTECT ADJACENT PRIVATE AND PUBLIC PROPERTY. THE COST (INCLUDING AN INITIAL MOBILIZATION AMOUNT) AND ANY FINES ASSESSED TO THE CITY SHALL BE CHARGED TO THE OWNER OF THE PROJECT.
- NECESSARY MATERIALS TO IMPLEMENT THE REQUIRED BMPS SHALL BE AVAILABLE ON SITE TO FACILITATE RAPID DEPLOYMENT OR TO REPAIR ANY BMP FAILURES. 4. CITY STAFF SHALL BE ALERTED BY THE CONTRACTOR, PERMITTEE OR OWNER, AS NEEDED FOR EMERGENCY
- WORK DURING STORMS. RUN-ON FLOW ONTO THE SITE SHALL BE PROPERLY MANAGED AND PLANNED FOR TO PREVENT FAILURE OF
- BMPS AND/OR ILLEGAL DISCHARGES FROM THE PROJECT SITE INTO THE STORM DRAIN. 6. STORM DRAIN INLET PROTECTION SHALL BE INSTALLED AT EVERY ONSITE STORM DRAIN INLET TO PREVENT SEDIMENT FROM ENTERING THE STORM DRAIN SYSTEM. WHERE FEASIBLE DESILTING BASINS SHALL ALSO BE
- PROVIDED AT DRAINAGE OUTLETS FROM THE GRADED SITE. EROSION CONTROL MEASURES SHALL BE IMPLEMENTED ON SLOPES AND ANY EXPOSED SOIL USING THE FOLLOWING BMPS, FIBER BLANKETS, BONDED FIBER MATRIX; OR BY INSTALLING OR MAINTAINING EXISTING
- VEGETATION. THE CONTRACTOR SHALL IMMEDIATELY REPAIR AND STABILIZE ANY ERODED AREAS. INACTIVE SLOPES SHALL BE PROTECTED AND STABILIZED. ALL EXPOSED SOIL INCLUDING INACTIVE AND ACTIVE SLOPES SHALL BE PROTECTED PRIOR TO A RAIN EVENT.
- 8. ALL UNPAVED GRADED CHANNELS SHALL IMPLEMENT EROSION PREVENTION MEASURES SUCH AS, LINING AND INSTALLING VELOCITY CHECK DAMS AT REGULAR INTERVALS.
- 9. STREET SWEEPING VEHICLES WITH VACUUMS AND WATER TANKS SHALL BE USED TO KEEP PAVED STREETS FREE OF LOOSE SOIL AND/OR CONSTRUCTION DEBRIS.
- 10. CONTRACTORS SHALL HAVE WATER TRUCKS AND EQUIPMENT ON-SITE TO MINIMIZE AIRBORNE DUST CREATED FROM GRADING AND HAULING OPERATIONS OR EXCESSIVE WIND CONDITIONS. ADDITIONAL DUST CONTROL MEASURES SHALL BE IMPLEMENTED AD NEEDED.
- 11. STOCKPILES SHALL BE COVERED AT THE END OF EACH WORKING DAY AND PRIOR TO FORECAST RAIN. ASPHALT SHALL ADDITIONALLY BE PLACED ON A LAYER OF PLASTIC SHEET, OR EQUIVALENT.
- 12. ALL PORTABLE TOILETS SHALL HAVE SECONDARY CONTAINMENT AND NOT BE LOCATED NEAR STORM DRAIN (I.E., CATCH BASIN OR STREET).
- 13. VEHICLES SHALL HAVE DRIP PANS UNDERNEATH THEM AND ANY LEAKS OR SPILLS SHALL BE PROMPTLY REPAIRED AND REMOVED.
- 14. ALL DEBRIS SHALL BE PLACED IN DUMPSTERS WITH LIDS. THE LIDS SHALL BE CLOSED AT THE END OF EACH DAY AND ARE NOT TO BE OVERFILLED. ADDITIONAL TRASH PICK-UPS SHALL BE MADE AS NECESSARY.
- 15. LIQUID MATERIALS SHALL BE STORED IN CLOSED CONTAINERS IN SECONDARY CONTAINMENT AND UNDER COVER. SOLID MATERIALS SHALL BE STORED ON PALLETS AND BE COVERED PRIOR TO FORECAST RAIN.
- 16. A MATERIALS WASHOUT SHALL BE AVAILABLE ONSITE WHENEVER LIQUID MATERIALS ARE USED. THE WASHOUT SHALL FULLY CONTAIN WASH MATERIALS AND THE SURROUNDING AREA SHALL BE KEPT FREE OF SPILLS.
- 17. DISCHARGE OF POTABLE WATER (SUCH AS FROM POWER WASHING OR FILLING WATER TRUCKS) SHALL BE PREVENTED OR DIRECTED TO LANDSCAPE.
- 18. PERIMETER CONTROL IS REQUIRED ON ALL SITES. 19. ALL ACTIVE ENTRANCES SHALL PREVENT TRACKING BY INSTALLING STABILIZED CONSTRUCTION ENTRANCES.

SHEET INDEX

TITLE	SHEET 1
DEMOLITION	SHEET 2
GRADING	SHEET 3
EROSION CONTROL	SHEET 4

LEGAL DESCRIPTION:

BLOCK 262 LOT 1

SITE ADDRESS: 1160 S CITRUS AVE

ESCONDIDO, CA 92027

ASSESSOR'S PARCEL NO .: APN: 231-220-21

ENGINEER'S CERTIFICATION OF STRUCTURAL BMP'S

THE ENGINEER OF RECORD SHALL VERIFY THAT THE STRUCTURAL BMP'S HAVE BEEN CONSTRUCTED AND OPERATE IN COMPLIANCE WITH ALL OF THE DESIGN SPECIFICATIONS, PLANS, PERMITS, ORDINANCES AND THE REQUIREMENTS OF THE MS4 PERMIT.

THE ENGINEER OF RECORD SHALL PROVIDE THE FIELD OFFICE WITH A SIGNED AND STAMPED CERTIFICATION(S) THAT THE PROJECT'S SITE DESIGN AND STRUCTURAL BMP'S WERE INSTALLED IN ACCORDANCE WITH THE APPROVED PLANS AND SWOMP. THE CERTIFICATION SHALL INCLUDE PHOTOGRAPHS TAKEN DURING SEVERAL PHASES OF THE TREATMENT FACILITIES DURING CONSTRUCTION (INCLUDING PHOTOGRAPHS OF SUBSURFACE STRUCTURES AND MATERIALS) AND FINAL AS-BUILT CONDITIONS.

RETAINING WALL NOTES

- 1. ALL RETAINING WALLS SHALL COMPLY WITH THE LATEST EDITION OF THE CALIFORNIA BUILDING CODES WHICH TYPICALLY ADOPTS THE LATEST UBC "UNIFORM BUILDING CODE".
- 2. THE PROJECT ENGINEER SHALL PROVIDE THE FOLLOWING INSPECTION REPORTS AND/OR CERTIFICATIONS TO THE FIELD ENGINEERING INSPECTOR DURING RETAINING WALL CONSTRUCTION:
- AFTER RETAINING WALL FOUNDATION EXCAVATION AND PRIOR TO STEEL PLACEMENT, THE SOILS ENGINEER SHALL CERTIFY IN WRITING THAT THE FOUNDATION EXCAVATIONS COMPLY WITH THE INTENT OF THE SOILS REPORT.
- ALL SPECIAL INSPECTION CERTIFICATIONS AS CALLED FOR ON THESE PLANS. 3. TWO (2) COPIES OF A RETAINING WALL CERTIFICATION REPORT SIGNED AND SEALED BY A CALIFORNIA REGISTERED CIVIL ENGINEER SHALL BE SUBMITTED TO THE FIELD ENGINEERING INSPECTOR PRIOR TO ROUGH GRADING SIGN-OFF. THE REPORT SHALL CERTIFY THAT ALL CONSTRUCTION MATERIALS (SIZE

THE REPORT SHALL GERTIN						
SPACING, STRENGTH, ETC.)	ARE IN	ACCORDANCE	WITH	THESE /	APPROVED	PLANS.

SOILS ENGINEER CERTIFICAT THIS GRADING AND RETAINING WALL PLAN THE UNDERSIGNED AND FOUND TO BE IN RECOMMENDATIONS AND SPECIFICATIONS REPORT PREPARED FOR THIS DEVELOPMEN COMPANY: ADDRESS: PHONE: ENGINEER:	N HAS BEEN REVIEWED BY CONFORMANCE WITH THE OUTLINED IN THE SOILS		CAL 1	IGALERT	ENRICO-JOAQUIN JUMP STREET EN	GINEERIN	ig inc.	
CONSTRUCTION RECORD	REFERENCES	Date	By	REV	ISIONS	App'd	Date	
Contractor								DESCRIPTION:
						1		

EARTHWORK DATA

 $CUT = 802 ext{ C.Y. FILL} = 78 ext{ C.Y.}$ $NET = 60 ext{ C.Y. REMEDIAL} = 0 ext{ C.Y.}$

TOTAL LOT AREA = TOTAL DISTURBED AREA =

THESE QUANTITIES DO NOT INCLUDE ANY LOSSES DUE TO SHRINKAGE. SUBSIDENCE. OVEREXCAVATION. OR ANY SPECIAL REQUIREMENTS THAT MAY BE SPECIFIED IN THE PRELIMINARY SOILS REPORT. THESE QUANTITIES ARE FOR PERMIT PURPOSES ONLY. ALL CONTRACTORS BIDDING ON THIS PROJECT SHOULD MAKE THEIR OWN DETERMINATION OF EARTHWORK QUANTITIES PRIOR TO SUBMITTING A BID.

DECLARATION OF RESPONSIBLE CHARGE

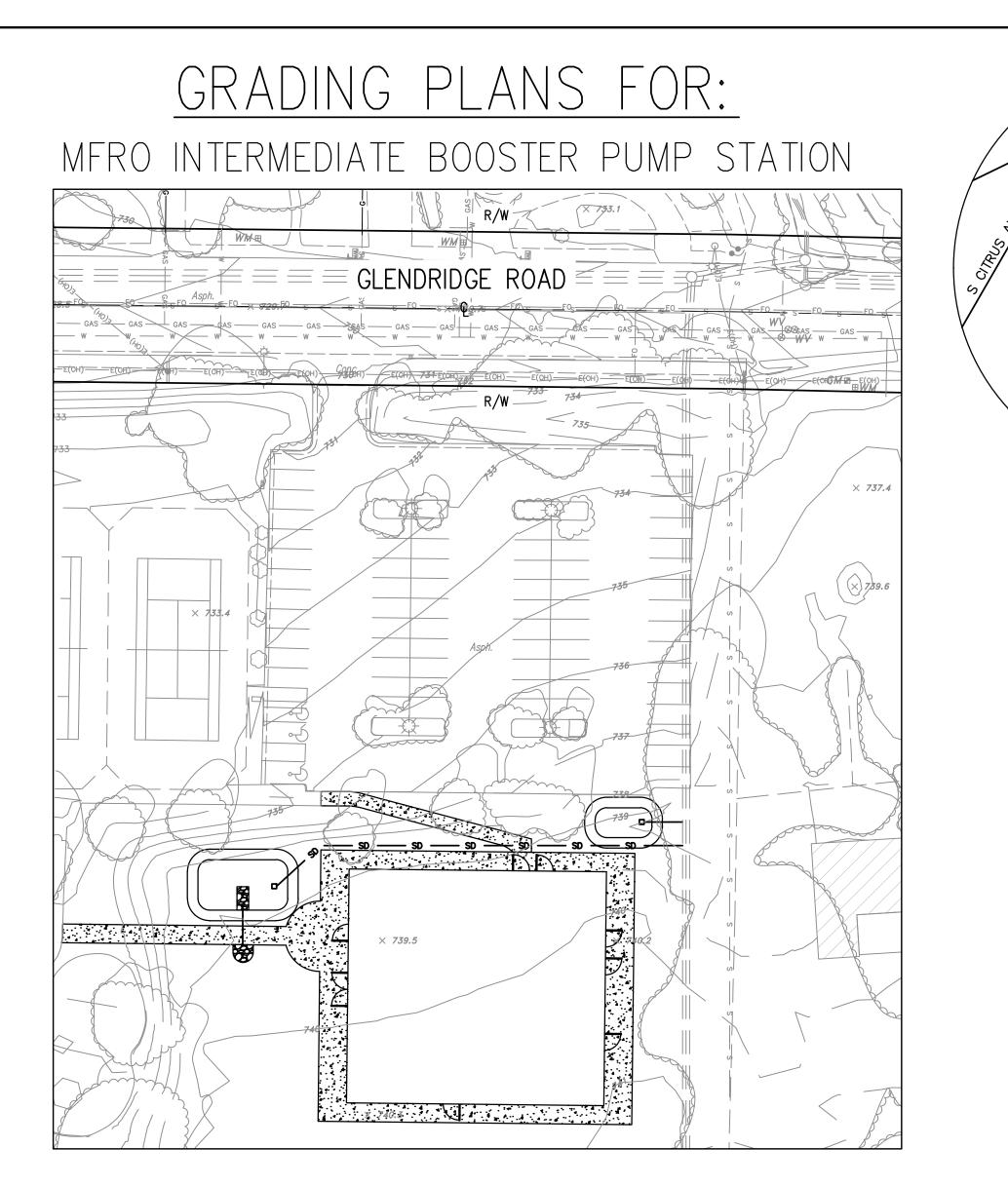
I HEREBY DECLARE THAT I AM THE ENGINEER OF WORK FOR THIS PROJECT. THAT I HAVE EXERCISED RESPONSIBLE CHARGE OVER THE DESIGN OF THE PROJECT AS DEFINED IN SECTION 6703 OF THE BUSINESS AND PROFESSIONS CODE, AND THAT THE DESIGN IS CONSISTENT WITH CURRENT STANDARDS.

I UNDERSTAND THAT THE CHECK OF PROJECT DRAWINGS AND SPECIFICATIONS BY THE CITY OF ESCONDIDO IS CONFINED TO A REVIEW ONLY AND DOES NOT RELIEVE ME, AS ENGINEER OF WORK, OF MY RESPONSIBILITIES FOR PROJECT DESIGN.

5	
2	
2	

Inspector_

Date Completed_



OUTSIDE OF BUILDING MAX. CUT DEPTH _____ [FT] MAX CUT SLOPE RATIO (2:1MAX) _____ MAX. FILL DEPTH _____ [FT] MAX FILL SLOPE RATIO (2:1MAX) _____

DATE

<u>KEY MAP</u> 1"=40'

> OWNER/APPLICANT <u>DEVELÓPER:</u>

CITY OF ESCONDIDO	
NAME 201 NORTH BROADWAY	
ADDRESS ESCONDIDO, CA 92025	
CITY, STATE, ZIP CODE (760) 839 – 4651	
PHONÉ	DAVID V. CAR

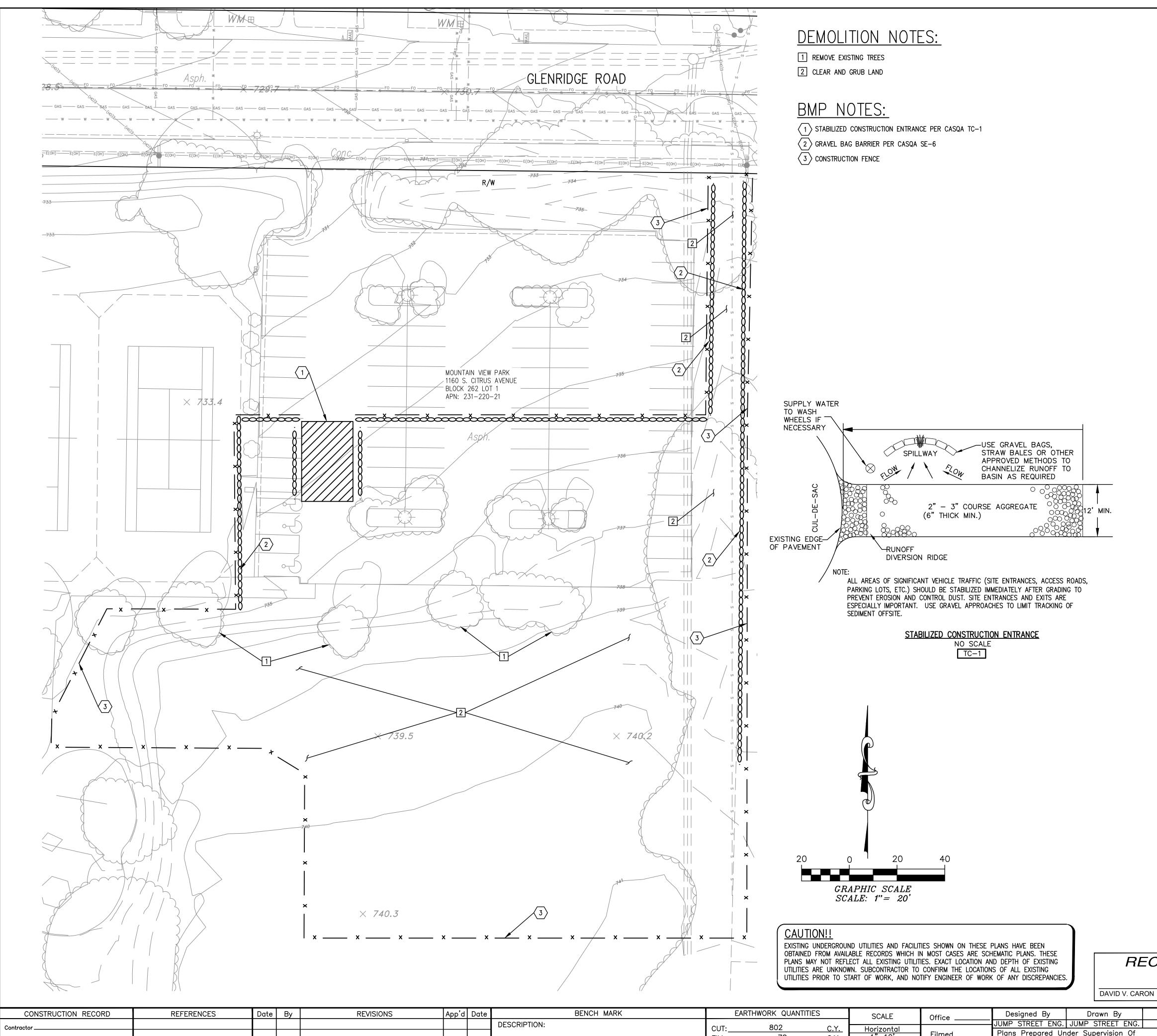
Date _

R.C.E. No. XXXXX

BENCH MARK		EARTHWORK QUANTITIES			SCALE	Office	Designed By	Drawn By
DESCRIPTION:						ennee	JUMP STREET ENG.	JUMP STREET ENG.
DESORT HON:		CUT:	802	C.Y	Horizontal			
		FILL:	78	C.Y.	1"=10'	Filmed	Plans Prepared Un	ider Supervision Of
		BALANCE:	60	C.Y.	Vertical	Τ		
Bench Mark No.	Elevation				N./A.	Traffic		

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PAGE 1130 E1					L GRADING, DRAINAGE ANTING OF PERMANEN	AND STORM WATER TREA T LANDSCAPING.	ATMENT FACILITIES,
VICINITY MAP		<u>LEGEND</u>					
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		SIDEWALK UNI	DERDRAIN		SDRSD D-27		
	G	4" YARD INLE STABILIZED CC SILTATION FEN MASONRY RET RADING NOT	DNSTRUCTION NCE TAINING WAL		CASQA TC-1 CASQA SE-1 SDRSD C-01		
	1.	GRADING, THE	CONTRACTO	OR SHALL OBTAIN		ger than 48 hours b 'Rom the engineering fi	
	2.		S SHOWN C	ON THESE PLANS	IN ACCORDANCE WIT	H ARTICLE 55, EXCAVATIO	ON AND GRADING, OF
	3.	THE ESCONDID	PORT PREPA	ARED BY DATED _	3/26/2021	AND ALL SUPPLEMEN	TS THEREOF ARE
	4.	All Fills Si by a pad Cef grading inspe compaction r engineering g the applicat engineering g	HALL BE CC RTIFICATION ECTION FOF REPORT SH REOLOGIC AS BLE CONDI REOLOGIST'S	REPORT, SHALL R ALL FILLS EXO ALL INCLUDE A S SPECTS OF THE O TIONS OF THE RECOMMENDATIO	0% OF MAXIMUM DEI BE SUBMITTED TO THE CEEDING ONE FOOT IN STATEMENT THAT TH GRADING HAVE BEEN IN GRADING PERMIT, TH NS. THE COMPACTION	NSITY. A COMPACTION R FIELD ENGINEER PRIC DEPTH, FOR EACH GRA E GEOTECHNICAL ENGINE NSPECTED AND ARE IN C HE GEOTECHNICAL ENGINE REPORT AND PAD CE PRIOR TO THE ROUGH C	OR TO THE ROUGH ADED PAD. THE ERING AND OMPLIANCE WITH ER'S AND RTIFICATION LETTER
	5.		ADJACENT N			CORNERS AND TO BLEND 066-C OF THE GRADING	
	6. 7.	ALL SLOPES O HEIGHT, THE	VER THREE CONTRACTO	R SHALL PROVIDI	E PERMANENT SPRINKL	ED. FOR SLOPES OVER LER SYSTEMS INSTALLED CH PAD A MINIMUM OF 3	ON EACH LOT.
	7.	REQUESTING I BY A CALIFO 33965 OR LES	FINAL ROUG ORNIA LICE S, AND MUS	H GRADING INSPE NSED LAND SUF ST CONTAIN AN E	ECTION. THE CERTIFICA RVEYOR OR REGISTER	TE MUST BE AN ORIGINAL RED CIVIL ENGINEER WITH NEAREST TENTH OF (L SIGNED AND SEALED AN RCE NO. OF
	8.				BLASTING PERMIT IS KWY. AT (760) 839-5	OBTAINED FROM THE ESC 5400.	ONDIDO FIRE
RECORD DF	RAWIN	G	By(f Comme	APPR(Date eer)	City of Choice	•
V. CARON R.C.E. X	XXXXX_	DATE		FIELD ENGINE PRIOR TO	EERING OFFICE GRADING		City Project No.
ENG.	CITY of	ESCONDIE	0		ENGINEERI	ING SERVICES	Drawing No.
Of	MFRO	INTERME		BOOST	ER PUMP	STATION	GPXX-XXXX

Sheet 1 of 4

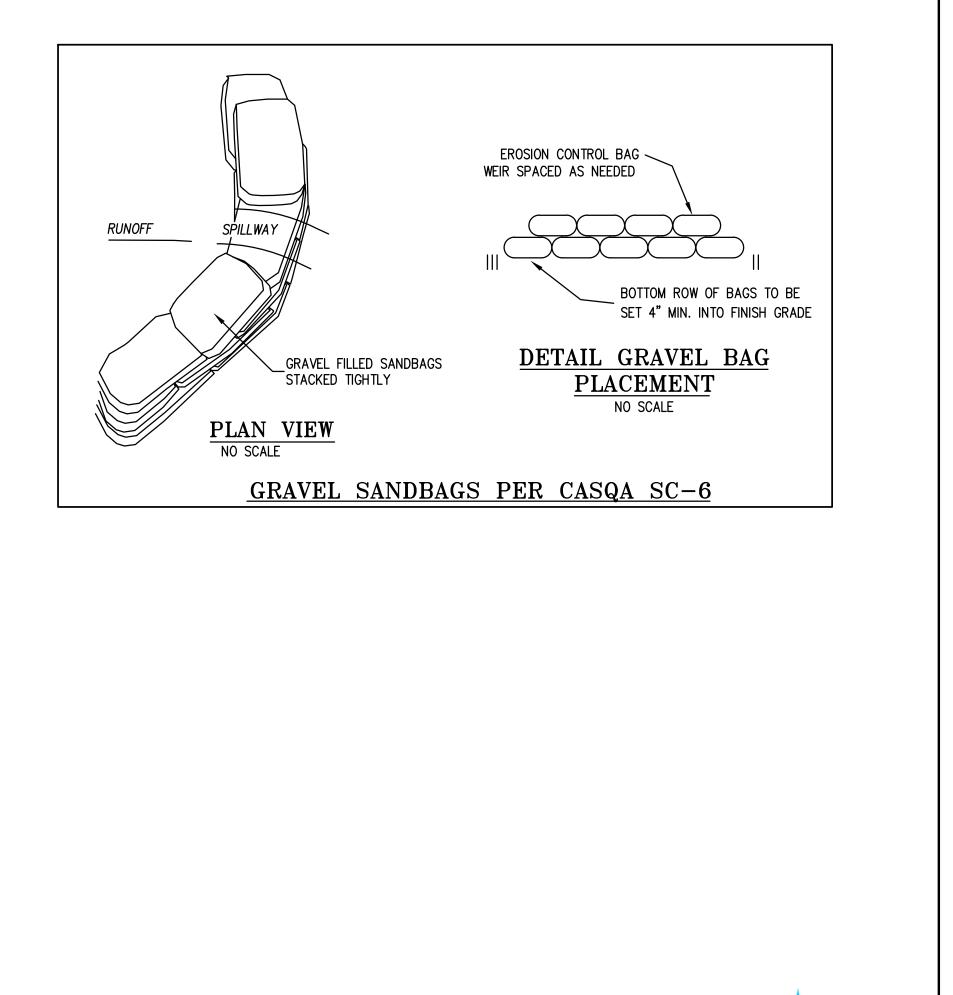


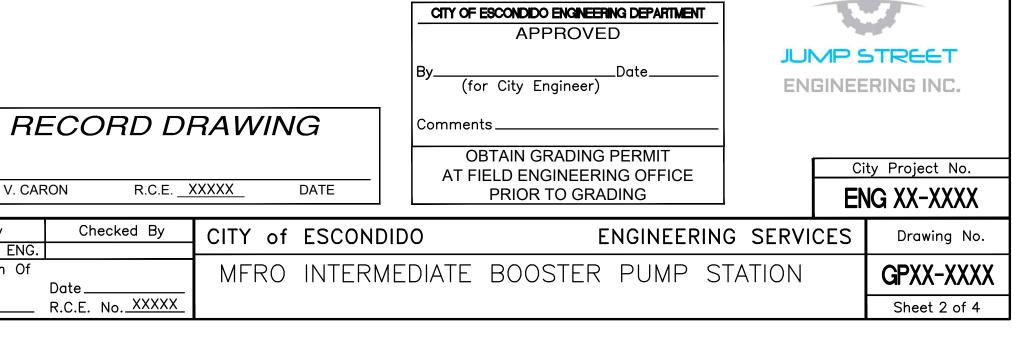
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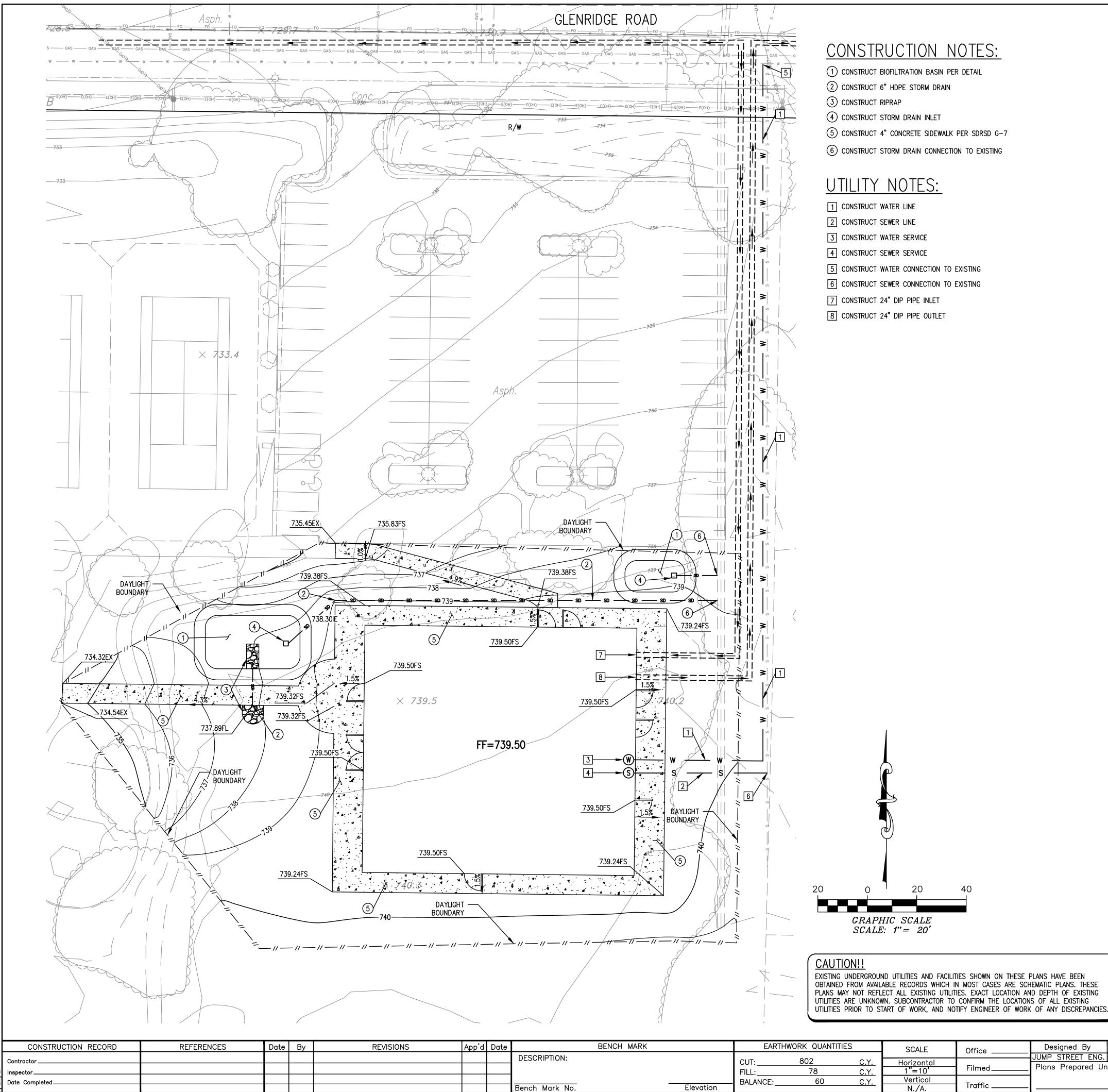
Date Completed_

BENCH MARK	EARTHWORK QUANTITIES	SCALE	Office	Designed By Drawn By	
	CUT: <u>802 C.Y.</u> FILL: 78 C.Y.	Horizontal 1"=10'	Filmed	JUMP STREET ENG. JUMP STREET ENG. Plans Prepared Under Supervision Of	
lo. Elevation	BALANCE: 60 C.Y.	Vertical N./A.	Traffic		Da R.(

DEMOLITION PLAN

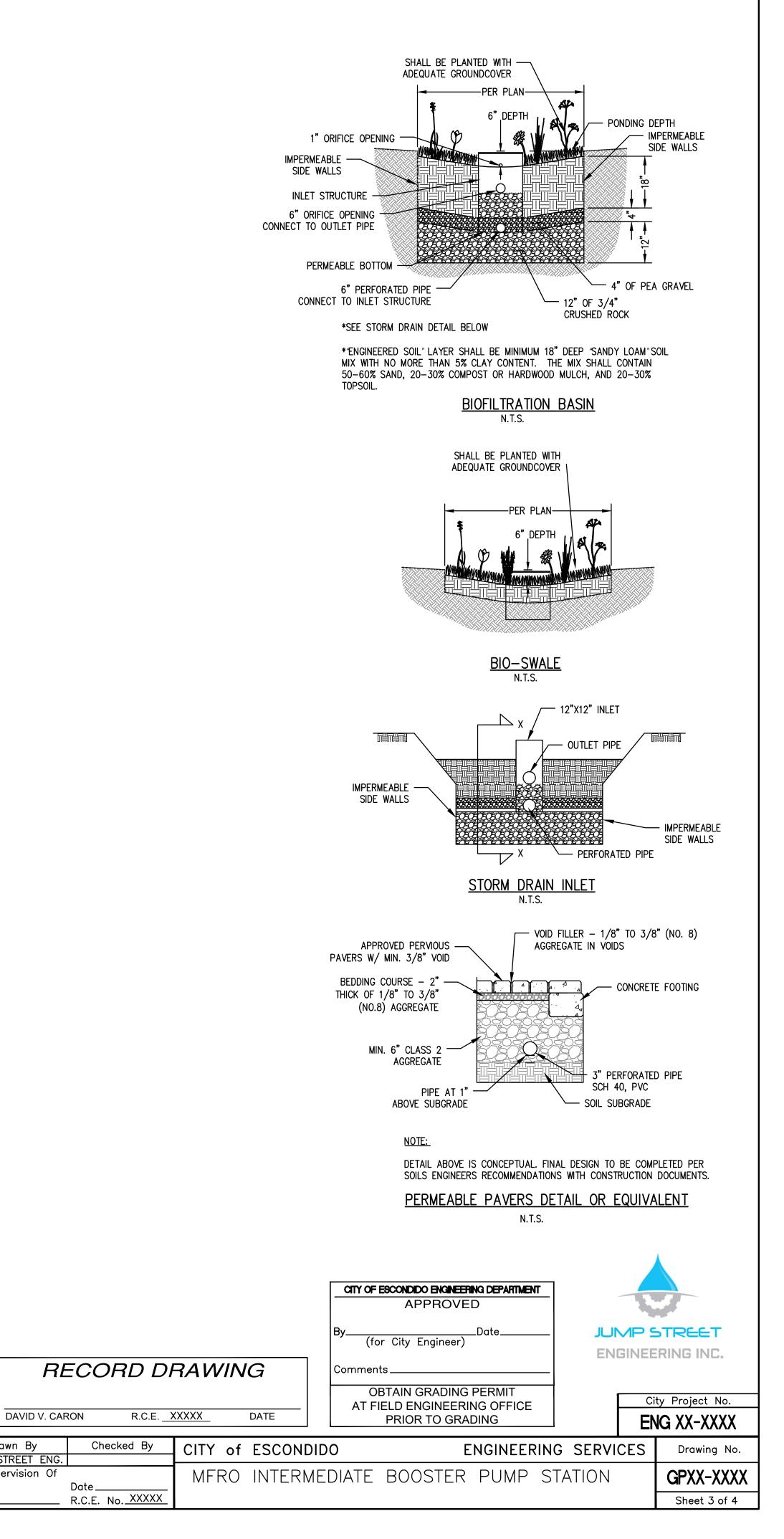


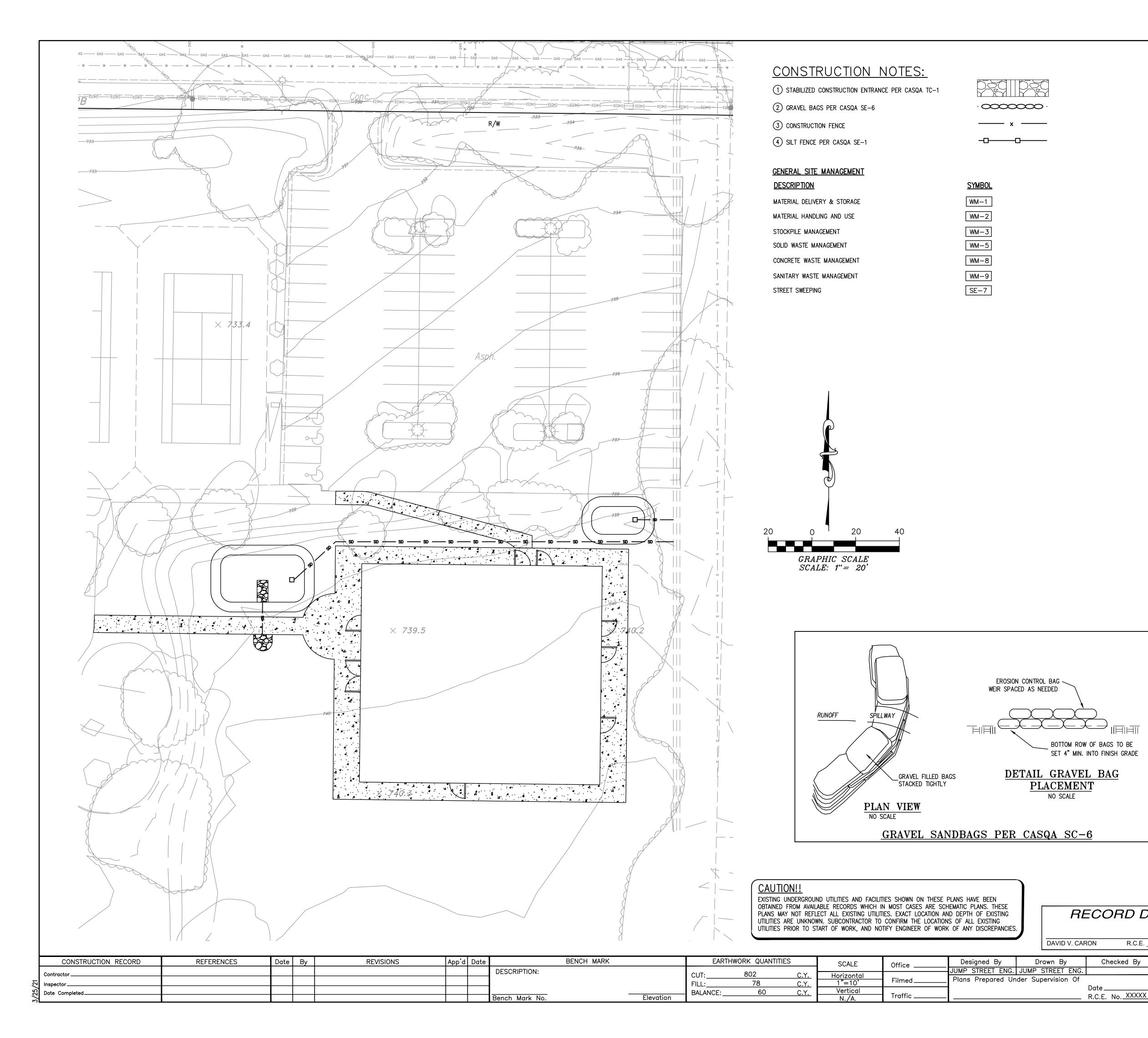




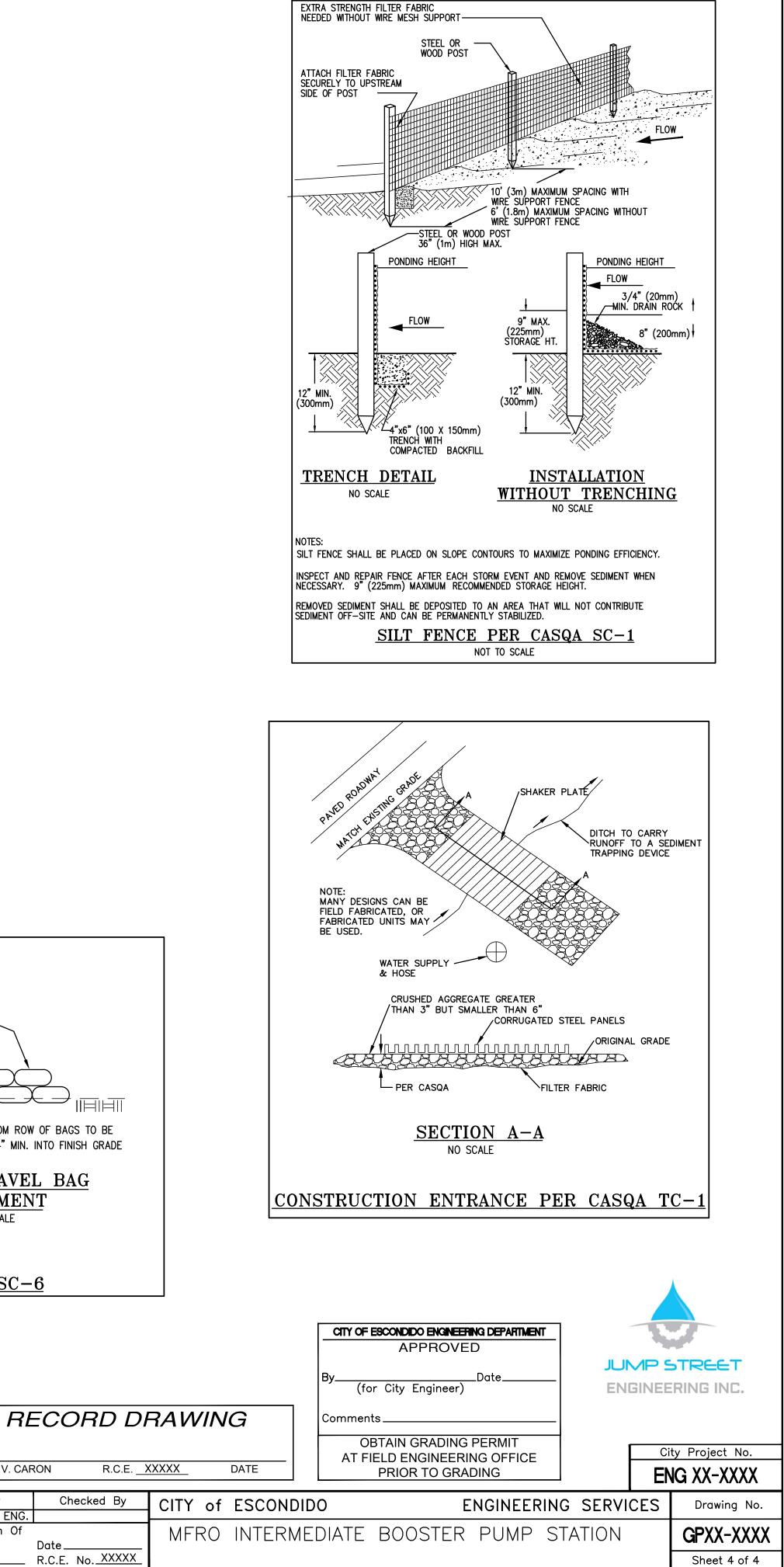
BENCH MARK	EARTHWORK QUANTITIES		SCALE	Office	Designed By	Drawn By	
No. Elevation	CUT: 802 FILL: 78 BALANCE: 60	<u>C.Y.</u> <u>C.Y.</u> <u>C.Y.</u>	Horizontal 1"=10' Vertical N./A.	Filmed	JUMP STREET ENG. Plans Prepared Un	JUMP STREET ENG. der Supervision Of	DR

PRELIMINARY GRADING PLAN





EROSION CONTROL PLAN



Appendix A - Structural Assumptions

TABLE 1607.1 MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS, L., AND MINIMUM CONCENTRATED LIVE LOADS⁹

OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (lbs.)
1. Apartments (see residential)		
2. Access floor systems Office use	50 100	2,000 2,000
Computer use 3. Armories and drill rooms	100 ^m	2,000
4. Assembly areas Fixed seats (fastened to floor)	60 ^m	
Follow spot, projections and control rooms Lobbies Movable seats Stage floors Platforms (assembly) Other assembly areas	50 100 ^m 100 ^m 150 ^m 100 ^m	_
5. Balconies and decks ^h	Same as occupancy served	_
6. Catwalks	40	300
7. Cornices	60	
8. Corridors First floor Other floors	100 Same as occupancy served except as indicated	_
9. Dining rooms and restaurants	100 ^m	—
10. Dwellings (see residential)		
 Elevator machine room and control room grating (on area of 2 inches by 2 inches) 	_	300
12. Finish light floor plate construction (on area of 1 inch by 1 inch)	_	200
13. Fire escapes On single-family dwellings only	100 40	—
14. Garages (passenger vehicles only) Trucks and buses	40 ^m Note a See Section 1607.7	
15. Handrails, guards and grab bars	See Section 1607.8	
16. Helipads	See Se	ection 1607.6
 Hospitals Corridors above first floor Operating rooms, laboratories Patient rooms 	80 60 40	1,000 1,000 1,000
18. Hotels (see residential)		—
19. Libraries Corridors above first floor Reading rooms Stack rooms	80 60 150 ^{b, m}	1,000 1,000 1,000
20. Manufacturing Heavy Light	250 ^m 125 ^m	3,000 2,000
21. Marquees, except one- and two- family dwellings	75	_
22. Office buildings Corridors above first floor File and computer rooms shall be designed for heavier loads based on anticipated occupancy Lething and first flow more ident.	80 —	2,000
Lobbies and first-floor corridors Offices (continuea	100 50	2,000 2,000

TABLE 1607.1—continued MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS, L_o, AND MINIMUM CONCENTRATED LIVE LOADS⁹

AND MINIMUM CONCENTRATED LIVE LOADS ⁹		
OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (lbs.)
23. Penal institutions Cell blocks Corridors	40 100	_
 24. Recreational uses: Bowling alleys, poolrooms and similar uses Dance halls and ballrooms Gymnasiums Ice skating rink Reviewing stands, grandstands and bleachers Roller skating rink Stadiums and arenas with fixed seats (fastened to floor) 	75 ^m 100 ^m 250 ^m 100 ^{c, m} 100 ^{c, m}	_
 25. Residential One- and two-family dwellings Uninhabitable attics without storageⁱ Uninhabitable attics with storage^{i,j,k} Habitable attics and sleeping areas^k Canopies, including marquees All other areas Hotels and multifamily dwellings Private rooms and corridors serving them Public rooms^m and corridors serving them 	10 20 30 20 40 40	_
 26. Roofs All roof surfaces subject to maintenance workers Awnings and canopies: Fabric construction supported by a skeleton structure All other construction, except one-and two-family dwellings Ordinary flat, pitched, and curved roofs (that are not occupiable) Primary roof members exposed to a work floor: Single panel point of lower chord of roof trusses or any point along primary structural members supporting roofs over manufacturing, storage warehouses, and repair garages All other primary roof members Occupiable roofs: Roof gardens Assembly areas All other similar areas 	5 Nonreducible 20 20 20 100 100 ^m Note 1	300 2,000 300 Note 1
27. Schools Classrooms Corridors above first floor First-floor corridors	40 80 100	1,000 1,000 1,000
28. Scuttles, skylight ribs and accessible ceilings	_	200
29. Sidewalks, vehicular driveways and yards, subject to trucking	250 ^{d, m}	8,000 ^e
(continued)		

(continued)

TABLE 1607.1—continued MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS, *L*,, AND MINIMUM CONCENTRATED LIVE LOADS⁹

OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (lbs.)
30. Stairs and exits One- and two-family dwellings All other	40 100	300 ^f 300 ^f
31. Storage warehouses (shall be designed for heavier loads if required for anticipated storage) Heavy Light	250 ^m 125 ^m	_
32. Stores Retail First floor Upper floors Wholesale, all floors	100 75 125 ^m	1,000 1,000 1,000
33. Vehicle barriers	See Section 1607.8.3	
34. Walkways and elevated platforms (other than exitways)	60	_
35. Yards and terraces, pedestrians	100 ^m	—
36. [OSHPD 2] Storage racks and wall-hung cabinets.	Total loads ⁿ	_

For SI: 1 inch = 25.4 mm, 1 square inch = 645.16 mm^2 ,

1 square foot = 0.0929 m^2 , 1 pound per square foot = 0.0479 kN/m^2 , 1 pound = 0.004448 kN, 1 pound per cubic foot = 16 kg/m^3 .

- a. Floors in garages or portions of buildings used for the storage of motor vehicles shall be designed for the uniformly distributed live loads of this Table or the following concentrated loads: (1) for garages restricted to passenger vehicles accommodating not more than nine passengers, 3,000 pounds acting on an area of 4¹/₂ inches by 4¹/₂ inches; (2) for mechanical parking structures without slab or deck that are used for storing passenger vehicles only, 2,250 pounds per wheel.
- b. The loading applies to stack room floors that support nonmobile, doublefaced library book stacks, subject to the following limitations:
 - 1. The nominal book stack unit height shall not exceed 90 inches;
 - 2. The nominal shelf depth shall not exceed 12 inches for each face; and
 - 3. Parallel rows of double-faced book stacks shall be separated by aisles not less than 36 inches wide.
- c. Design in accordance with ICC 300.
- d. Other uniform loads in accordance with an approved method containing provisions for truck loadings shall be considered where appropriate.
- e. The concentrated wheel load shall be applied on an area of 4.5 inches by 4.5 inches.
- f. The minimum concentrated load on stair treads shall be applied on an area of 2 inches by 2 inches. This load need not be assumed to act concurrently with the uniform load.
- g. Where snow loads occur that are in excess of the design conditions, the structure shall be designed to support the loads due to the increased loads caused by drift buildup or a greater snow design determined by the building official (see Section 1608).
- h. See Section 1604.8.3 for decks attached to exterior walls.
- i. Uninhabitable attics without storage are those where the maximum clear height between the joists and rafters is less than 42 inches, or where there are not two or more adjacent trusses with web configurations capable of accommodating an assumed rectangle 42 inches in height by 24 inches in width, or greater, within the plane of the trusses. This live load need not be assumed to act concurrently with any other live load requirements.
- j. Uninhabitable attics with storage are those where the maximum clear height between the joists and rafters is 42 inches or greater, or where there are two or more adjacent trusses with web configurations capable of accommodating an assumed rectangle 42 inches in height by 24 inches in width, or greater, within the plane of the trusses.

The live load need only be applied to those portions of the joists or truss bottom chords where both of the following conditions are met:

 The attic area is accessible from an opening not less than 20 inches in width by 30 inches in length that is located where the clear height in the attic is a minimum of 30 inches; and (continued) TABLE 1607.1—continued MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS, L_o, AND MINIMUM CONCENTRATED LIVE LOADS⁹

ii. The slopes of the joists or truss bottom chords are no greater than two units vertical in 12 units horizontal.

The remaining portions of the joists or truss bottom chords shall be designed for a uniformly distributed concurrent live load of not less than 10 pounds per square foot.

- k. Attic spaces served by stairways other than the pull-down type shall be designed to support the minimum live load specified for habitable attics and sleeping rooms.
- Areas of occupiable roofs, other than roof gardens and assembly areas, shall be designed for appropriate loads as approved by the building official. Unoccupied landscaped areas of roofs shall be designed in accordance with Section 1607.12.3.
- m. Live load reduction is not permitted unless specific exceptions of Section 1607.10 apply.
- n. [OSHPD 2] The minimum vertical design live load shall be as follows: Paper media:

12-inch-deep (305 mm) shelf 33 pounds per lineal foot (482 N/m)

- 15-inch-deep (381 mm) shelf 41 pounds per lineal foot (598 N/m), or
- 33 pounds per cubic foot (5183 N/m^3) per total volume of the rack or cabinet, whichever is less.

Film media:

18-inch-deep (457 mm) shelf 100 pounds per lineal foot (1459 N/m), or
50 pounds per cubic foot (7853 N/m³) per total volume of the rack or cabinet, whichever is less.

Other media:

20 pounds per cubic foot (311 N/m³) or 20 pounds per square foot (958 Pa), whichever is less, but not less than actual loads.

1607.3 Uniform live loads. The live loads used in the design of buildings and other structures shall be the maximum loads expected by the intended use or occupancy but shall in no case be less than the minimum uniformly distributed live loads given in Table 1607.1.

1607.4 Concentrated live loads. Floors and other similar surfaces shall be designed to support the uniformly distributed live loads prescribed in Section 1607.3 or the concentrated live loads, given in Table 1607.1, whichever produces the greater load effects. Unless otherwise specified, the indicated concentration shall be assumed to be uniformly distributed over an area of $2^{1}/_{2}$ feet by $2^{1}/_{2}$ feet (762 mm by 762 mm) and shall be located so as to produce the maximum load effects in the structural members.

1607.5 Partition loads. In office buildings and in other buildings where partition locations are subject to change, provisions for partition weight shall be made, whether or not partitions are shown on the construction documents, unless the specified live load is 80 psf (3.83 kN/m^2) or greater. The partition load shall be not less than a uniformly distributed live load of 15 psf (0.72 kN/m^2) .

1607.6 Helipads. Helipads shall be designed for the following live loads:

- 1. A uniform live load, *L*, as specified below. This load shall not be reduced.
 - 1.1. 40 psf (1.92 kN/m²) where the design basis helicopter has a maximum take-off weight of 3,000 pounds (13.35 kN) or less.
 - 1.2. 60 psf (2.87 kN/m²) where the design basis helicopter has a maximum take-off weight greater than 3,000 pounds (13.35 kN).

Material Weights

Coverings

Hardwood (1" nominal)	4.0psf	
Quarry or Ceramic Tile, 3/4"	10.0psf	
Linoleum or Soft Tile	1.5psf	
Vinyl Tile 1/8" thick	1.4psf	
Soft Tile and Sheet	1.5psf	
GypCrete 3/4"	6.5psf	

Concrete:

Regular, 1" thick	12.0psf
Reinforced (1-1/2" thick)	17.5psf
Lightweight (1-1/2" thick)	12.5psf
Terrazo (1-1/2" thick)	19.0psf
Cement Finish (per inch thick)	12.0psf

Walls/Partitions

Masonry

Brick 4" thick	38.0psf
Concrete Block 12" thick	90.0psf
Cinder Concrete Block 12" thick	90.0psf
Stucco, 7/8" thick	10.0psf
Hollow Clay Tile (load bearing)	23.0psf
Hollow Clay Tile (nonbearing)	18.0psf
Hollow Gypsum Block 8" thick	26.0psf
Limestone	55.0psf
Terra-cotta Tile	25.0psf
Stone	55.0psf
8" Concrete Wall	100.0 psf
Wood Paneling, 1"	2.5psf

Material Weights

Metal Grid System	0.8psf
Metal Suspension with Tile	1.8psf
Wood Suspension with Tile	2.5psf
Plaster, 1"	8.0psf
Plaster on wood lath, 1"	10.0psf
Plaster on metal lath, 1"	8.5psf

Roofing

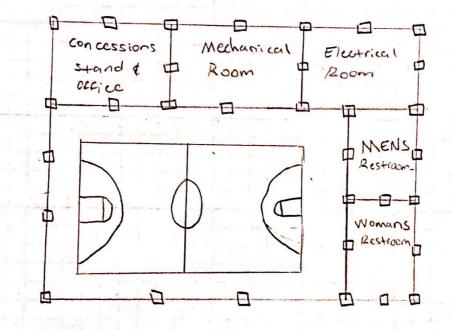
Asphalt Shingles	2.5psf
Wood Shakes	3.0psf
Roll Roofing	1.0psf
Asphalt Shingles, approx 1/4"	2.0psf
Cement asbestos shingles, 3/8"	4.0psf
Clay Tile (add 10psf for mortar)	9.0psf
Ludowici	10.0psf
Roman	12.0psf
Slate, 1/4"	10.0psf
Spanish	19.0psf
Wood, 1"	3.0psf
Cement Tile (add 6psf for mortar)	10.0psf
Corrugated Asbestos/Cement	4.0psf

Composition

235 lb Shingle & Paper	5.0psf
3-ply Ready Roofing	1.0psf
2-15 lb + 1-90lb	1.7psf
3-15 lb + 1-90lb	2.2psf
3-ply + Gravel	5.6psf
4-ply + Gravel	6.0psf
5-ply + Gravel	6.5psf

Appendix B - Structural Calculations

Column Location



Structural calculations

Live load :

 $\begin{array}{rcl} G_{\chi}m & : & 100 \ PSF & \times \ 90 \\ \hline & YmiP \ station \ Expans: 250 \ PSF & \chi \ 82x \ 38 = \ 779 \ 000 \ 1bs \\ \hline & Restrooms = \ 100 \ PSF & \chi \ 60 \\ \hline & Zo \\ \hline & SFrice \ / \ concentrated = \ 120, 000 \ 1bs \\ \hline & SFrice \ / \ concentrated = \ 2,000 \ 1bs \\ \hline & Concentrated = \ 2,000 \ 1bs \\ \hline & Total \ live \ load = \ 540,000 + \ 779,000 + \ 720,000 \\ \hline & 55,200 \ 1bs \end{array}$

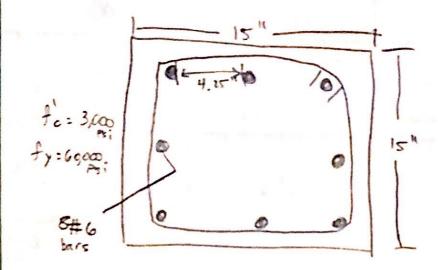
= 1, 494,200165 = 1; 494.2 Kips

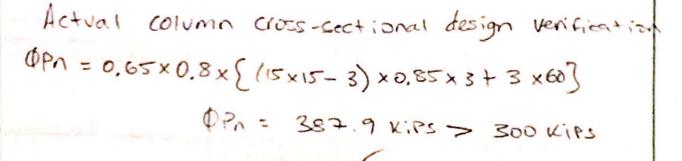
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Dead load: = Floor/ hardwood Flooring = 4 PSF × 90×60 = 21,600 lbs 1" reinforced regular weight concrete = 12, 5 PSF × 82×38 = 3895045 Carriet & Pad = 2 PSF × 28 × 38 = 2128 lbs 3/8" ceramic floor tile = 4.7 PSF × box 20 = 5,640 lbs ubilis 12" thick concrete blocks = 90 PSF x 25 x 300 = 675,000 bs 12" thick concrete blocks = 90 PSFX 14 × 248 = 312,480 lbs 1 12" thick concrete blocks = 90 PSF x = 312.48 K.PS 114× 14 = 143,640/65 = 143.64 Kirs Roof. < Spanish Tile = 19 PSF x 110 x 100 = 209,000 lbs = 209 Kips Total Dead load = 1408.4 Kips Column calcs PPn = 0.65 × 0.8 × [/15×15-As+) × 0.85×3 + Ast ×60] ≥ 209Km 401.92 = (225 - Ast) (2.55) + (Ast) (60) -171.83 < -2.55 Ast+ 60 Ast Ast 2 2.99 2 3 in2 1g = 3/15×15 = 1.33 %. -> within ACI code of 1im Steel bor selection: $3 = 8 \cdot \frac{T(\frac{x}{2})}{4} \rightarrow x = 0,477 = \frac{x^{2}}{44}$ 8776 bars are selected Clear spacing of bars = (15-5-2×6/8)= 4.25 in Use # 3 ties $S = \frac{1}{2} = \frac{1}{10} \times \frac{1}{6} = 12 \rightarrow S = 12 \text{ in}$ $S_2 = \frac{1}{48} \times \frac{2}{8} = 18$

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cross - section detail :





Appendix C - Column Locations

Appendix C - Column Locations

